



# *Flood-Frequency Analysis Updates for Bulletin 17B and the U.S. Geological Survey PeakFQ Program*

Presented at the National Hydraulic  
Engineering Conference

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# Flood-frequency estimation at gaged sites follows guidelines in Bulletin 17B



**Guidelines  
For  
Determining**

**Flood  
Flow  
Frequency**

Bulletin # 17B  
of the  
Hydrology Subcommittee

Revised September 1981  
Editorial Corrections March 1982

INTERAGENCY ADVISORY COMMITTEE  
ON WATER DATA



U.S. Department of the Interior  
Geological Survey  
Office of Water Data Coordination  
Reston, Virginia 22092

## History:

- 1967 Bulletin 15
- 1976 Bulletin 17
- 1977 Bulletin 17A
- 1981 Bulletin 17B
- 201? Bulletin 17C

[http://water.usgs.gov/osw/bulletin17b/dl\\_flow.pdf](http://water.usgs.gov/osw/bulletin17b/dl_flow.pdf)

# Bulletin 17B [1982, p. ii]

This present revision is adopted with the knowledge and understanding that review of these procedures will continue. When warranted by experience and by examination and testing of new techniques, other revisions will be published.

- Hydrologic Frequency Analysis Work Group (HFAWG) is tasked with examining and testing flood frequency methodology and providing guidance on revisions to Bulletin 17B

<http://acwi.gov/hydrology/Frequency/index.html>

# June 2013 HFAWG studies recommend changes in four main areas

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- 1) Historical information and weighted-moments approach
- 2) Low-outlier detection and treatment
- 3) Procedures for estimating generalized/regional skew
- 4) Procedures for estimating confidence limits

<http://acwi.gov/hydrology/Frequency/index.html>

# Key recommended revision to B17B

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- Adoption of the expected moments algorithm (EMA) framework for the analysis of data sets containing zeros, outliers, interval flow estimates, multiple thresholds, or historical and/or paleoflood information as the appropriate generalization of the method-of-moments to address such situations

<http://acwi.gov/hydrology/Frequency/index.html>

# EMA advantages

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- Extension of B17B LP3 method-of-moments approach, which includes a consistent statistical framework for ALL sources of information available
  - For simple cases with only a systematic record and a regional skew: EMA estimates = B17B estimates
- EMA deals with interval and multiple threshold data that conditional probability adjustment and historical weighted moments in B17B do not.
  - CSG and historic info are best described by intervals
- Provides confidence intervals that include skew uncertainty and reflect interval observations



# Other recommended revisions to B17B

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- Update with the new multiple Grubbs-Beck (MGB) test for the identification of Potentially Influential Low Flows (PILFs)
- Use of Bayesian Generalized Least Squares (BGLS) procedures for the derivation of regional skews for weighting with the at-site skews
- Correct uncertainty computations and confidence intervals
- Use of a multiple-threshold plotting position formula – compatible with EMA



# HFAWG testing to show new methods work

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- Theoretical arguments
  - Analytical results
  - Monte Carlo results
- Evaluation using real data
  - 82 test sites
  - Complicated situations
  - Judgment
- Recommendations and testing report published on Advisory Committee on Water Information website:
  - <http://acwi.gov/hydrology/Frequency/index.html>



# Bulletin 17B representation of peaks

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## ■ SYSTEMATIC PEAKS–

- Recorded during one or more periods of regular data collection;
- Can be from the continuous record or from a crest-stage gage

## ■ HISTORIC PEAKS-

- Records of floods that occurred outside the period of regular streamgaging
  - Discharge measurement during a flood event
  - Indirect measurement after a flood event

## ■ Peaks represented as a point or single value

# EMA representation of peaks

- Peaks are represented as intervals
- HISTORICAL – (systematic + historic peaks)
  - For every year  $Y$ , it is assumed that there was a peak discharge  $Q_Y$ , regardless of whether the discharge was recorded
- FLOW INTERVALS –  $(Q_{Y,lower}, Q_{Y,upper})$ 
  - Describes knowledge of peak flow  $Q_Y$  for every  $Y$
  - When peak is known with confidence,  $(Q_{Y,lower}, Q_{Y,upper}) = (Q_Y, Q_Y)$
  - Censored peaks:
    - Flow greater than some value  $Q_Y$  (Code G + X):
      - $(Q_{Y,lower}, Q_{Y,upper}) = (Q_Y, inf)$
    - Flow less than some value  $Q_Y$  (Code L):  $(Q_{Y,lower}, Q_{Y,upper}) = (0, Q_Y)$
  - Interval peak:
    - Flow greater than some value and less than another value

# EMA representation of peaks (cont.)

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- **PERCEPTION THRESHOLDS –  $(T_{Y,lower}, T_{Y,upper})$** 
  - Reflect range of flows that would have been measured/recorded had they occurred
  - Independent of actual peak discharges that have occurred
  - $T_{Y,lower}$  = smallest peak that would result in a recorded flow
  - $T_{Y,upper}$  = largest peak that would result in a documented flow
  - For periods of continuous, full-range streamgage record:  
 $(T_{Y,lower}, T_{Y,upper}) = (0, inf)$  where  $T_{Y,lower} = 0$  is the gage-base flow
  - Can adjust  $T_{Y,lower}$  to accommodate a changing gage-base flow

# EMA representation of data: continuous-record streamgauge



- From Peak Flow File:

<u>Date</u>	<u>Peak</u>	<u>Code</u>
1993-03-28	276	

- Flow Interval:

$$(Q_{1993,lower}, Q_{1993,upper}) = (276, 276)$$

- Perception Threshold:

$$(T_{1993,lower}, T_{1993,upper}) = (0, inf)$$

USGS 2011 Streamgaging Calendar, March, John A. Mazurek,  
April 20, 2009 <http://water.usgs.gov/osw/calendar.html>

# EMA representation of data: crest-stage gage



- Gage base in 1980: 20 cfs
  - CSG cannot record flow < 20 cfs
- From Peak Flow File (from USGS NWIS):

<u>Date</u>	<u>Peak</u>	<u>Code</u>
1980-10-20	20	4*

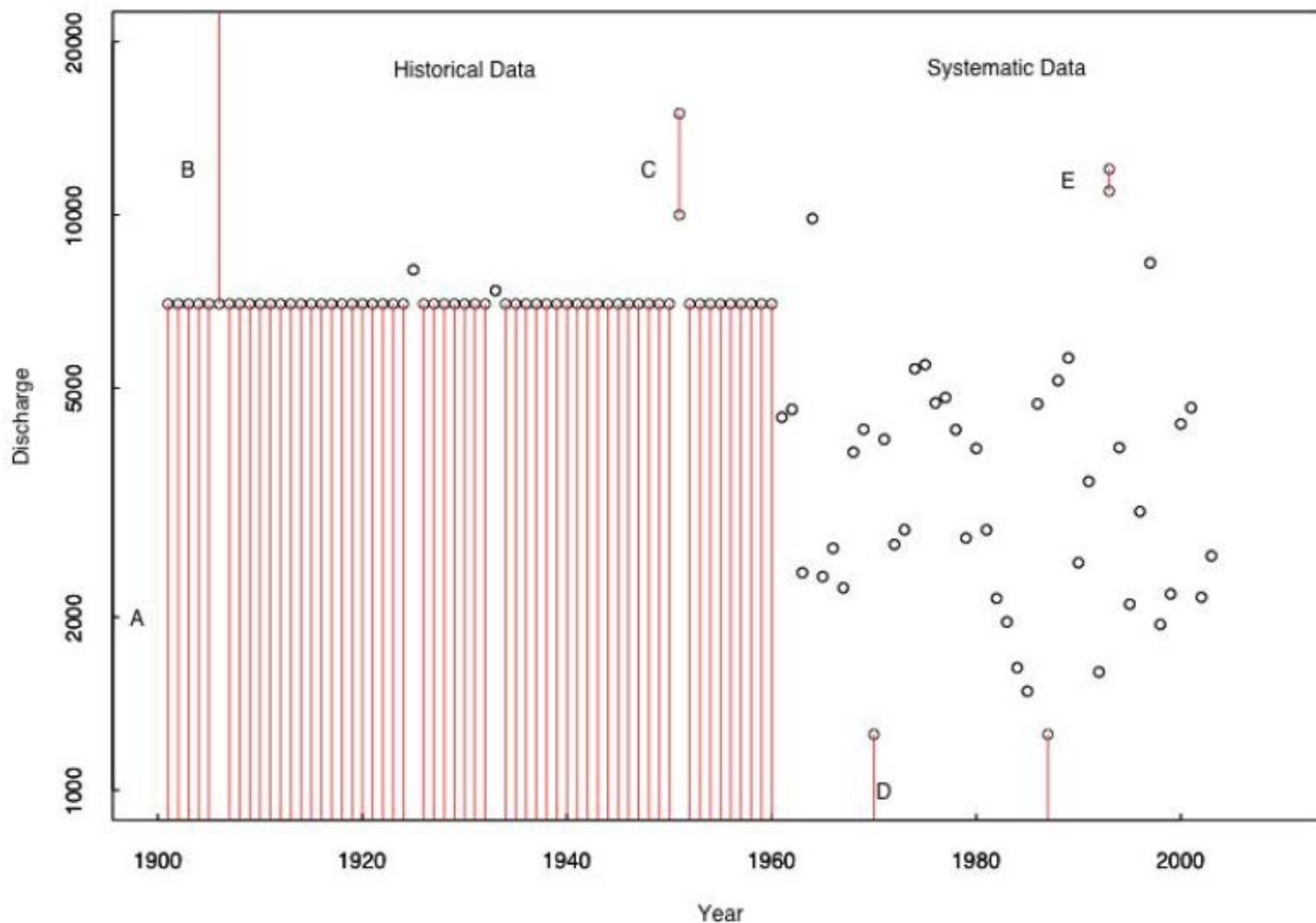
*\*Code 4: Discharge is less than indicated value, which is a minimum recordable discharge at this site*

- Flow Interval:  
 $(Q_{1980,lower}, Q_{1980,upper}) = (0, 20)$

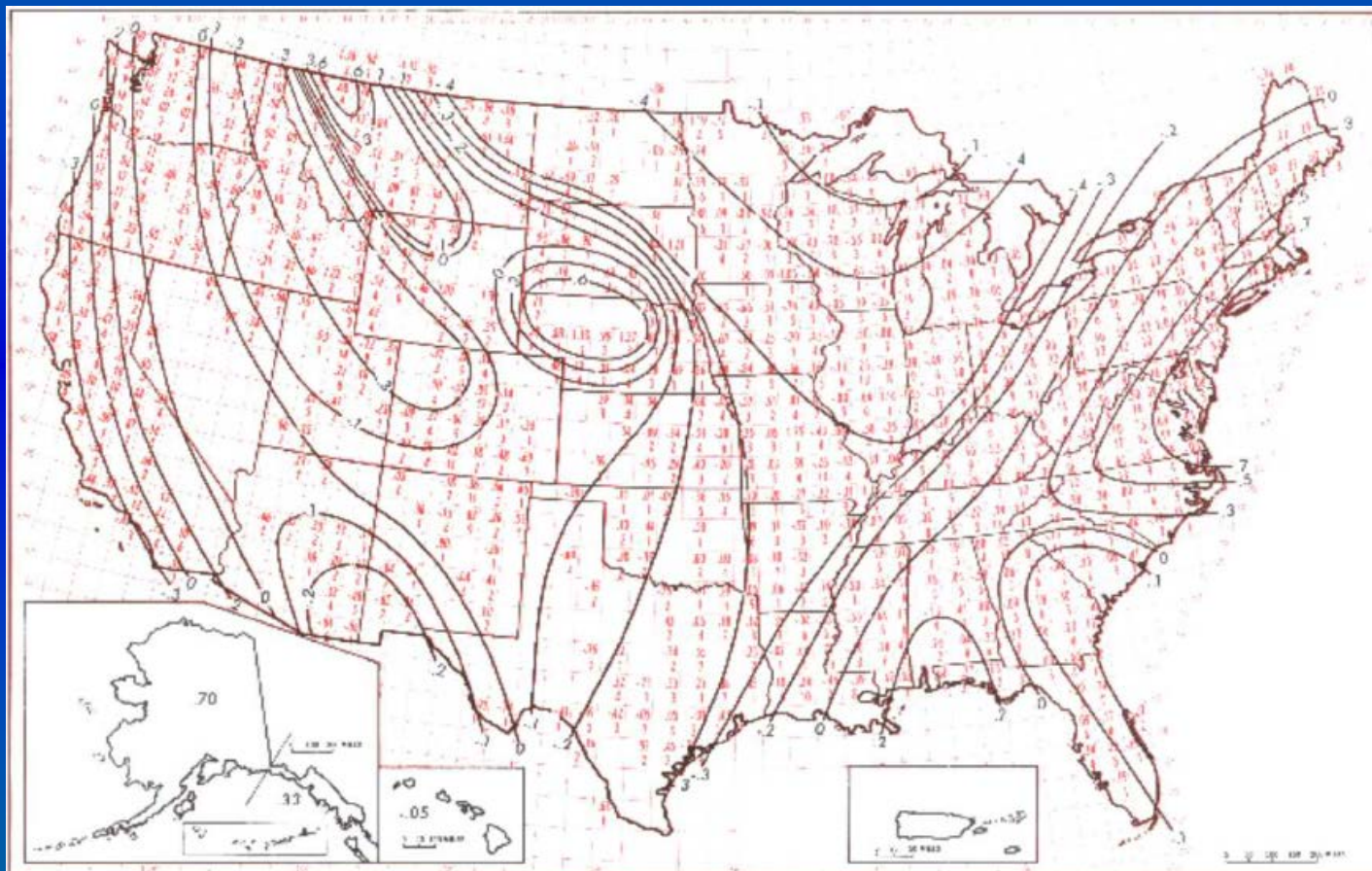
- Perception Threshold:  
 $(T_{1980,lower}, T_{1980,upper}) = (20, inf)$

Photo courtesy of Paul Rydlund.  
<http://mo.water.usgs.gov/surfwat/CSGWeb/index.htm>

# Types of interval data



# Estimating regional skew (1982 B17B)



**GENERALIZED SKEW COEFFICIENTS OF LOGARITHMS OF ANNUAL MAXIMUM STREAMFLOW**  
**AVERAGE SKEW COEFFICIENT BY ONE DEGREE QUADRANGLES**

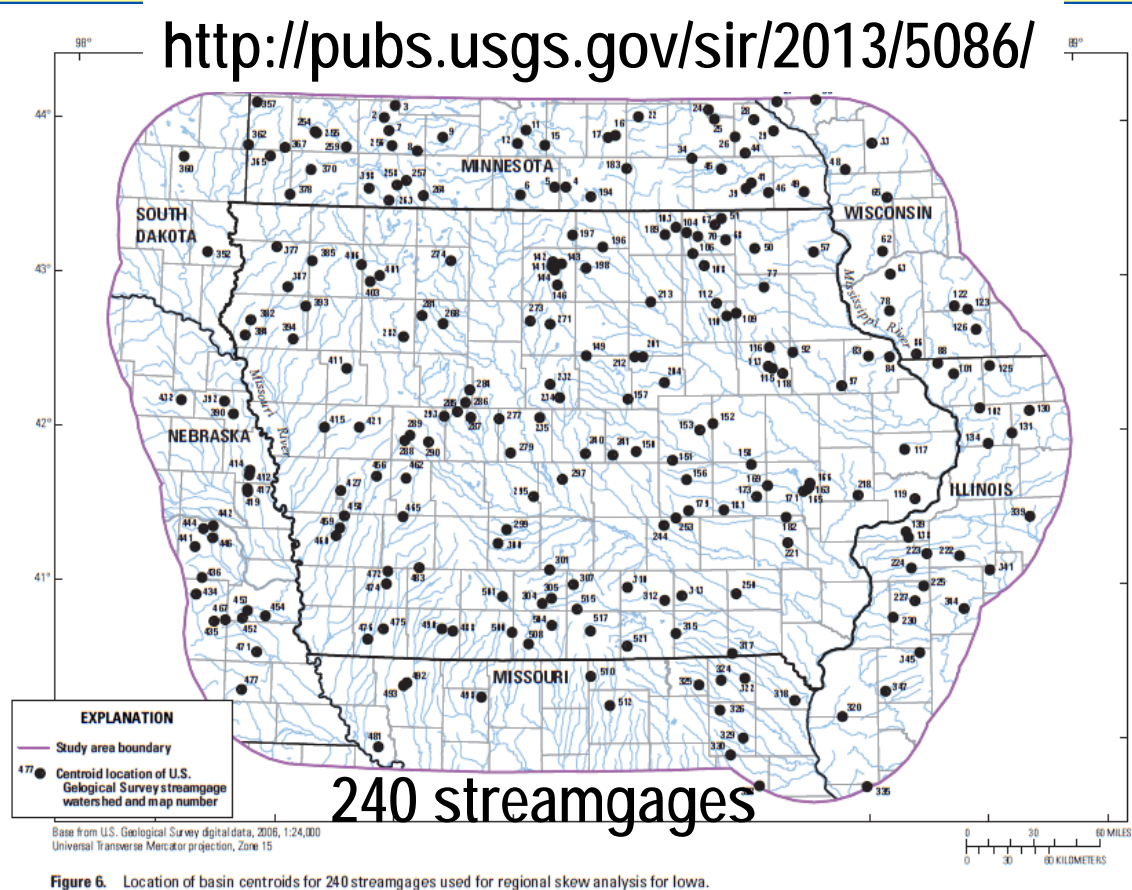
Lower number in each quadrangle is number of stream gaging stations for which the average shown above it was computed

# 2010 Iowa regional skew study

## Bayesian WLS/GLS regression

<http://pubs.usgs.gov/sir/2013/5086/>

Constant  
Regional  
Skew Value  
for Iowa =  
-0.400



1982 (MSE = 0.302)  
→ ERL = 17 yrs

2010 (MSE = 0.160)  
→ ERL = 50 yrs



# USGS PeakFQ flood-frequency analysis program (PKFQWin)

PeakFQ Version 7.1

File Help

Use File menu to Open PeakFQ data or PKFQWin spec file. Update Station, Threshold and Output specifications as desired. Click Run PeakFQ button to generate results.

PeakFQ Data File: C:\Users\laveilleux\Desktop\05464000.TXT

PeakFQ Spec File:

Station Specifications | Input/View | Output Options | Results

Global Analysis Option: B17B Global PILF (LO) Test Option: Single Grubbs-Beck

Station ID	Analysis Option	Beginning Year	Ending Year	Record Length	Inc Hist	Skew	Generalized	Gen Skew	Mean	Low Hist	PILF (LO)	PILF (LO) Test	High Sys	Hi-Outlier	Gage Base	Urban/Reg	Discharge	Peaks	Latitude	Longitude	Name
05464000	EMA	1929	2013	85	Yes	Weighted	-0.4	0.4	0.16	61000	0	Multiple	112000	0	0	No		42.49556	92.33417	05464000	

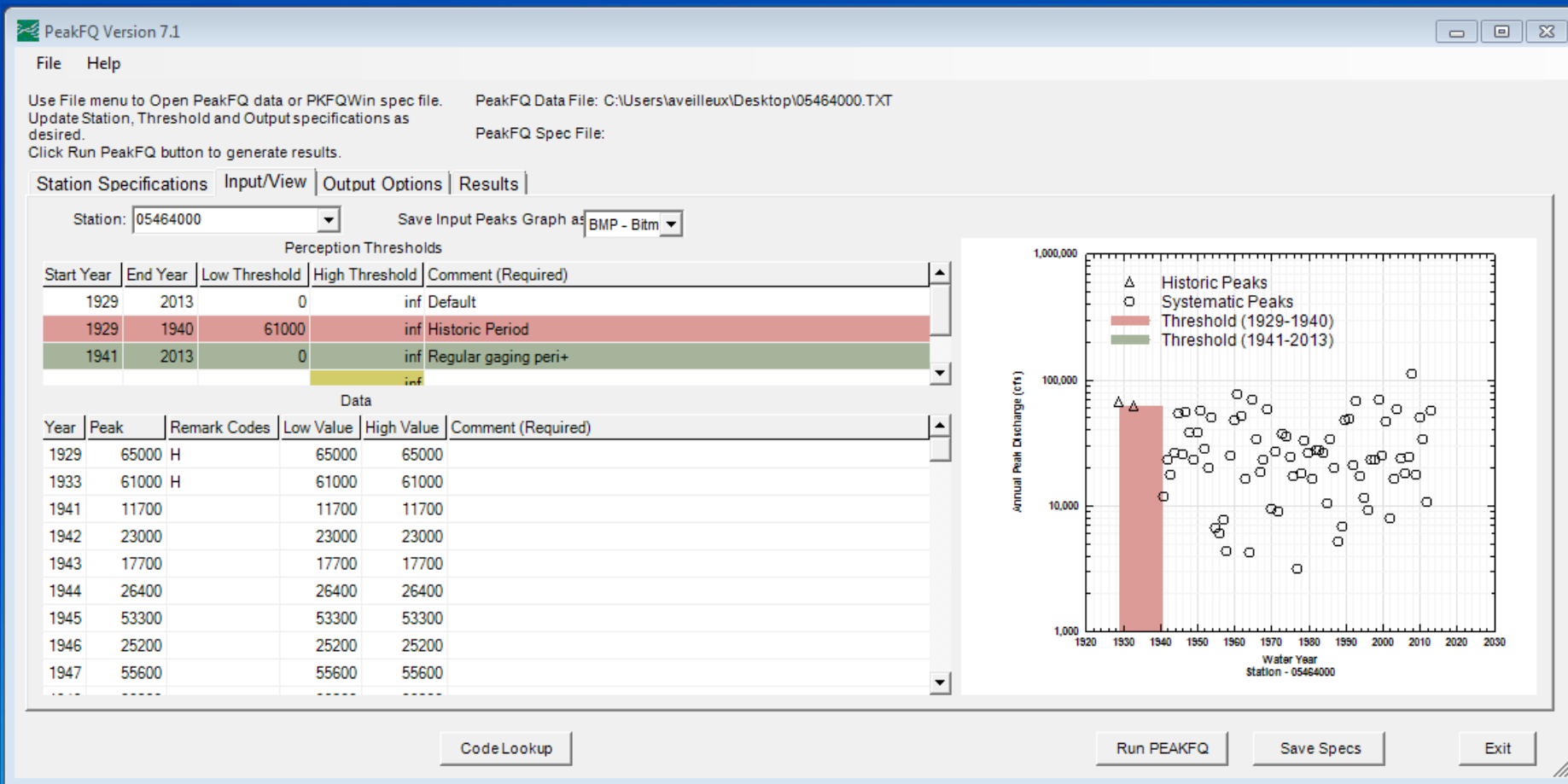
Run PEAKFQ Save Specs Exit

PeakFQ updated to Version 7.1 and released to public in May 2014

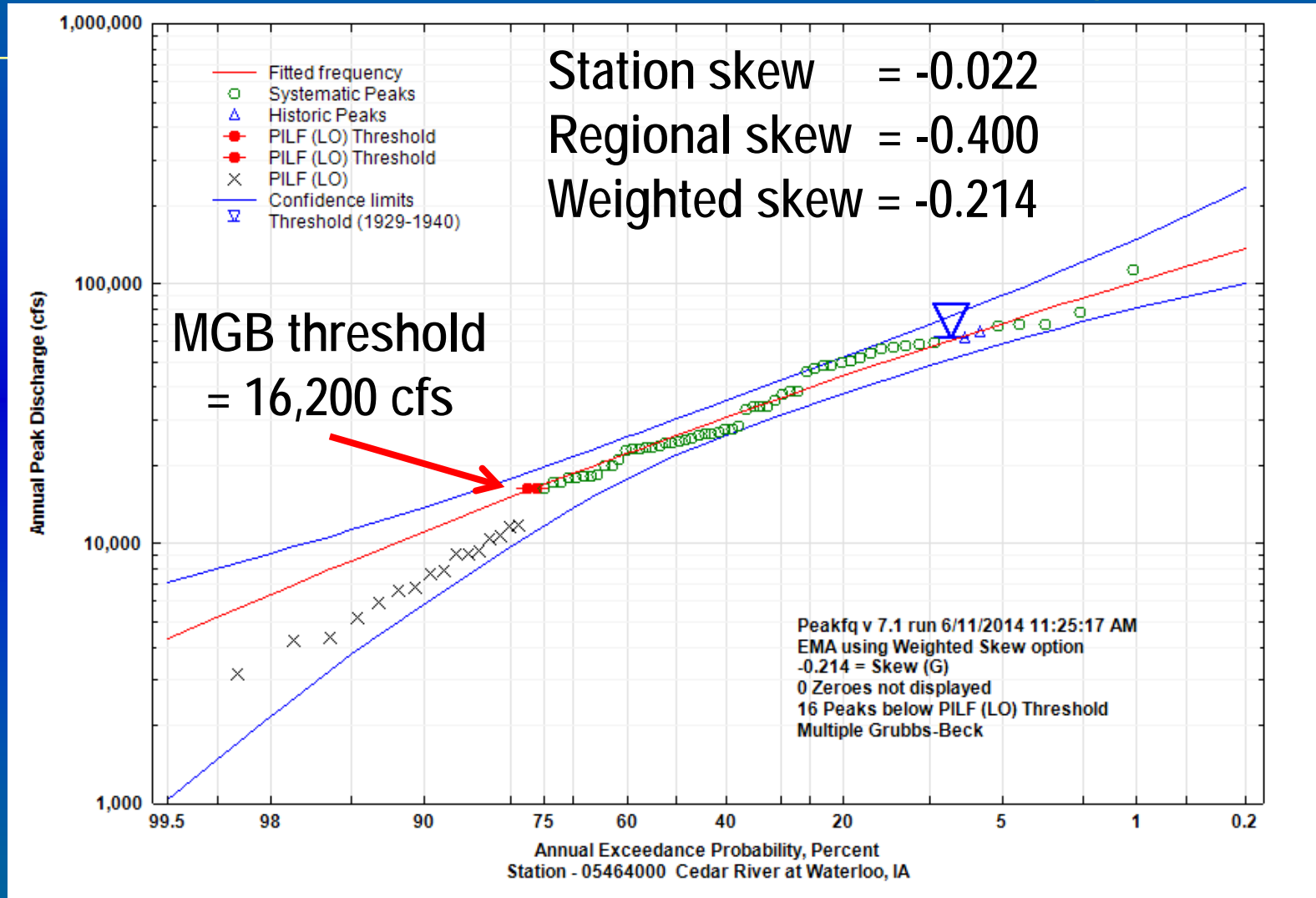
PeakFQ fact sheet: <http://pubs.usgs.gov/fs/2013/3108/>

# EMA perception thresholds

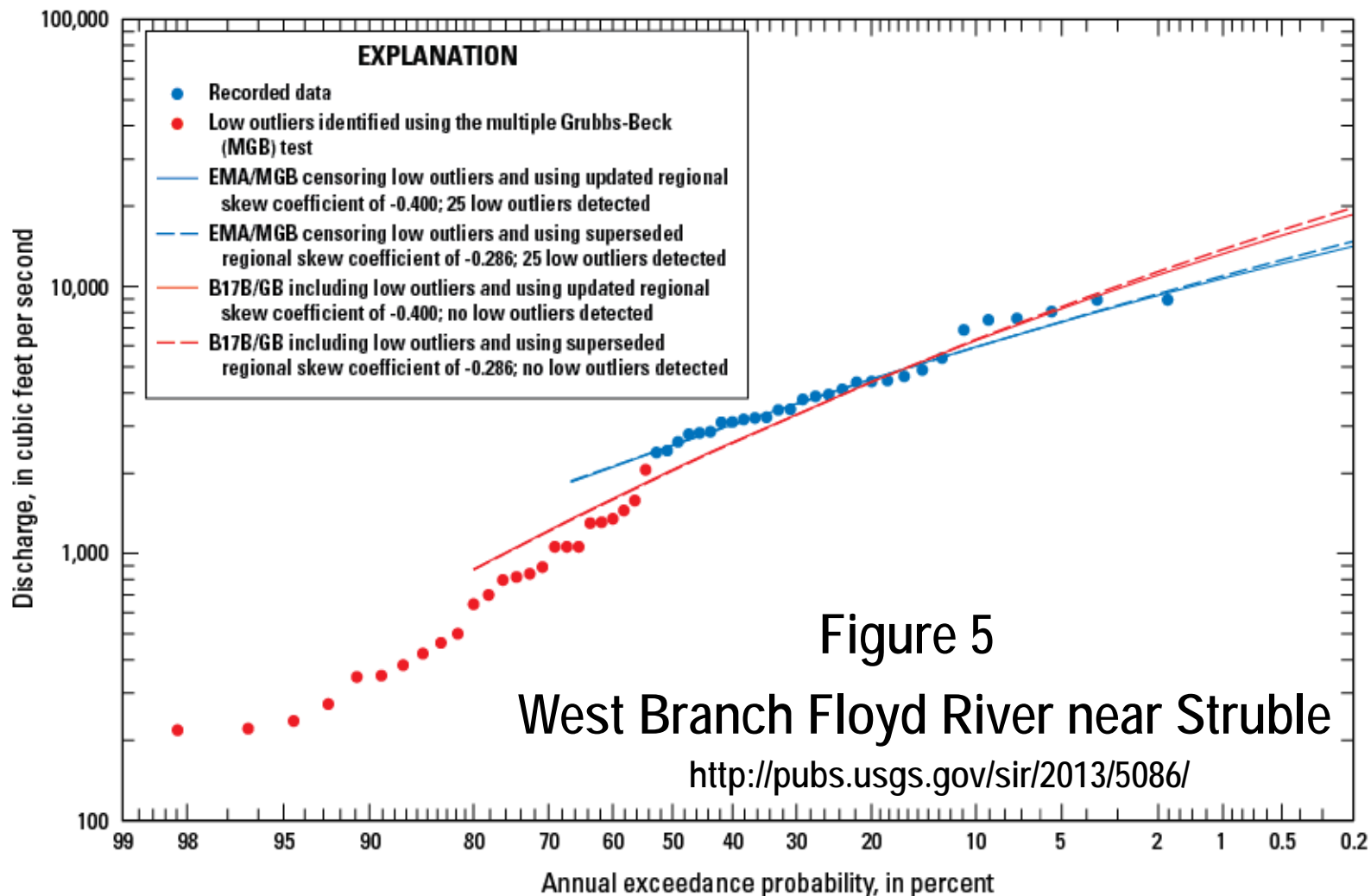
(missing years 1930-32, 1934-40 in historic period)



# PeakFQ output flood-frequency curve



# Effects of including or censoring low outliers



# PeakFQ output flood-frequency analysis results

Program PeakFq  
Version 7.1  
3/14/2014

U. S. GEOLOGICAL SURVEY  
Annual peak flow frequency analysis

Seq.001.001  
Run Date / Time  
06/11/2014 11:19

Station - 05464000 Cedar River at Waterloo, IA

## INPUT DATA SUMMARY

Number of peaks in record = 75  
Peaks not used in analysis = 0  
Systematic peaks in analysis = 73  
Historic peaks in analysis = 2  
Beginning Year = 1929  
Ending Year = 2013  
Historical Period Length = 85  
Generalized skew = -0.400  
Standard error = 0.400  
Mean Square error = 0.160  
Skew option = WEIGHTED  
Gage base discharge = 0.0  
User supplied high outlier threshold = --  
User supplied PILF (LO) criterion = --  
Plotting position parameter = 0.00  
Type of analysis = EMA  
PILF (LO) Test Method = MGBT  
Perception Thresholds:

Begin	End	Low	High	Comment
1929	2013	0.0	INF	DEFAULT
1929	1940	61000.0	INF	HISTORIC PERIOD
1941	2013	0.0	INF	REGULAR GAGING PERIOD: SYSTEMATIC RECORD

Interval Data = None Specified

\*\*\*\*\* NOTICE -- Preliminary machine computations. \*\*\*\*\*  
\*\*\*\*\* User responsible for assessment and interpretation. \*\*\*\*\*

EMA003I-PILFS (LOS) WERE DETECTED USING MULTIPLE GRUBBS-BECK TEST 16 16200.0  
THE FOLLOWING PEAKS (WITH CORRESPONDING P-VALUES) WERE CENSORED:  
3120.0 (0.2774)  
4210.0 (0.2252)  
4320.0 (0.0558)

MGB  
Threshold:  
16,200 cfs

# PeakFQ output flood-frequency analysis results

## Kendall's Tau Parameters

	TAU	P-VALUE	MEDIAN SLOPE	No. of PEAKS
SYSTEMATIC RECORD	0.008	0.924	4.522	73

Program PeakFq  
Version 7.1  
3/14/2014

U. S. GEOLOGICAL SURVEY  
Annual peak flow frequency analysis

Seq.001.002  
Run Date / Time  
06/11/2014 11:19

Station - 05464000 Cedar River at Waterloo, IA

## ANNUAL FREQUENCY CURVE PARAMETERS -- LOG-PEARSON TYPE III

### LOGARITHMIC

	MEAN	STANDARD DEVIATION	SKEW
EMA W/O REG. INFO	4.4074	0.2702	-0.022
EMA W/REG. INFO	4.4037	0.2778	-0.214

EMA ESTIMATE OF MSE OF SKEW W/O REG. INFO (AT-SITE) 0.0636

# PeakFQ output flood-frequency analysis results (cont.)

## ANNUAL FREQUENCY CURVE -- DISCHARGES AT SELECTED EXCEEDANCE PROBABILITIES

ANNUAL EXCEEDANCE PROBABILITY	EMA W/ REG INFO ESTIMATE	EMA W/O REG INFO ESTIMATE	<----- FOR EMA ESTIMATES ----->		
			VARIANCE OF EST.	95% CONFIDENCE LOWER	INTERVALS UPPER
0.9950	4292.	5080.	0.0268	1023.0	7076.0
0.9900	5179.	5950.	0.0201	1468.0	7959.0
0.9500	8523.	9147.	0.0084	3722.0	11210.0
0.9000	11020.	11490.	0.0050	5769.0	13660.0
0.8000	14900.	15150.	0.0027	9647.0	17640.0
0.6667	19610.	19580.	0.0016	14970.0	22780.0
0.5000	25920.	25610.	0.0012	21730.0	30060.0
0.4292	29020.	28610.	0.0011	24730.0	33720.0
0.2000	43640.	43160.	0.0012	37410.0	51760.0
0.1000	56590.	56630.	0.0016	47910.0	69400.0
0.0400	73960.	75580.	0.0023	61160.0	96220.0
0.0200	87470.	91020.	0.0031	70700.0	120100.0
0.0100	101400.	107600.	0.0041	79800.0	147900.0
0.0050	115700.	125300.	0.0054	88460.0	180500.0
0.0020	135400.	150600.	0.0075	99260.0	232600.0

# PeakFQ output flood-frequency analysis results (cont.)

## EMPIRICAL FREQUENCY CURVES -- HIRSCH-STEDINGER PLOTTING POSITIONS

WATER YEAR	RANKED DISCHARGE	EMA ESTIMATE	INTERVALS	
			LOW	HIGH
2008	112000.0	0.0103		
1961	76700.0	0.0206		
1965	69500.0	0.0309		
1999	69300.0	0.0412		
1993	68100.0	0.0515		
-1929	65000.0	0.0618		
-1933	61000.0	0.0721		
1969	58600.0	0.0956		
2004	58500.0	0.1089		
2013	57300.0	0.1222		
1951	56400.0	0.1355		
1947	55600.0	0.1488		
1945	53300.0	0.1621		
1962	51200.0	0.1754		
2010	49900.0	0.1887		
1954	49400.0	0.2020		
1991	48200.0	0.2153		
1960	48100.0	0.2286		
1990	47100.0	0.2419		
2001	45800.0	0.2552		
1950	38400.0	0.2685		
1948	38200.0	0.2818		
1973	37300.0	0.2951		
1974	35500.0	0.3084		
2011	33700.0	0.3217		
1966	33400.0	0.3483		
1986	33400.0	0.3350		
1979	32500.0	0.3616		
1952	28000.0	0.3749		
1982	27500.0	0.3882		
1983	27200.0	0.4015		
1971	26700.0	0.4148		
1944	26400.0	0.4281		
1984	26300.0	0.4414		
1980	25900.0	0.4547		
1946	25200.0	0.4680		
2000	24900.0	0.4813		
1959	24700.0	0.4946		
2007	24200.0	0.5079		

1975	24100.0	0.5212
2005	23700.0	0.5345
1997	23300.0	0.5611
1998	23300.0	0.5478
1949	23100.0	0.5744
1942	23000.0	0.5877
1968	22800.0	0.6010
1992	20900.0	0.6143
1953	19800.0	0.6276
1987	19700.0	0.6409
1967	18400.0	0.6542
2006	18100.0	0.6675
1978	18000.0	0.6808
1943	17700.0	0.7074
2009	17700.0	0.6941
1976	17200.0	0.7340
1994	17200.0	0.7207
1981	16300.0	0.7473
1963	16200.0	0.7739
2003	16200.0	0.7606
* 1941	11700.0	0.7872
* 1995	11600.0	0.8005
* 2012	10700.0	0.8138
* 1985	10400.0	0.8271
* 1970	9340.0	0.8404
* 1996	9100.0	0.8538
* 1972	9030.0	0.8671
* 2002	7820.0	0.8804
* 1957	7680.0	0.8937
* 1989	6730.0	0.9070
* 1955	6590.0	0.9203
* 1956	5920.0	0.9336
* 1988	5150.0	0.9469
* 1958	4320.0	0.9602
* 1964	4210.0	0.9735
* 1977	3120.0	0.9868

\* DENOTES PILF (LO)



# PeakFQ output flood-frequency analysis results (cont.)

## EMA REPRESENTATION OF DATA

WATER YEAR	OBSERVED		EMA		PERCEPTION THRESHOLDS	
	Q_LOWER	Q_UPPER	Q_LOWER	Q_UPPER	LOWER	UPPER
1929	65000.0	65000.0	65000.0	65000.0	61000.0	INF
1930	0.0	61000.0	0.0	61000.0	61000.0	INF
1931	0.0	61000.0	0.0	61000.0	61000.0	INF
1932	0.0	61000.0	0.0	61000.0	61000.0	INF
1933	61000.0	61000.0	61000.0	61000.0	61000.0	INF
1934	0.0	61000.0	0.0	61000.0	61000.0	INF
1935	0.0	61000.0	0.0	61000.0	61000.0	INF
1936	0.0	61000.0	0.0	61000.0	61000.0	INF
1937	0.0	61000.0	0.0	61000.0	61000.0	INF
1938	0.0	61000.0	0.0	61000.0	61000.0	INF
1939	0.0	61000.0	0.0	61000.0	61000.0	INF
1940	0.0	61000.0	0.0	61000.0	61000.0	INF
1941	11700.0	11700.0	0.0	16200.0	16200.0	INF
1942	23000.0	23000.0	23000.0	23000.0	16200.0	INF
1943	17700.0	17700.0	17700.0	17700.0	16200.0	INF
1944	26400.0	26400.0	26400.0	26400.0	16200.0	INF
1945	53300.0	53300.0	53300.0	53300.0	16200.0	INF
1946	25200.0	25200.0	25200.0	25200.0	16200.0	INF
1947	55600.0	55600.0	55600.0	55600.0	16200.0	INF
1948	38200.0	38200.0	38200.0	38200.0	16200.0	INF
1949	23100.0	23100.0	23100.0	23100.0	16200.0	INF
1950	38400.0	38400.0	38400.0	38400.0	16200.0	INF
1951	56400.0	56400.0	56400.0	56400.0	16200.0	INF
1952	28000.0	28000.0	28000.0	28000.0	16200.0	INF
1953	19800.0	19800.0	19800.0	19800.0	16200.0	INF
1954	49400.0	49400.0	49400.0	49400.0	16200.0	INF
1955	6590.0	6590.0	0.0	16200.0	16200.0	INF
1956	5920.0	5920.0	0.0	16200.0	16200.0	INF
1957	7680.0	7680.0	0.0	16200.0	16200.0	INF
1958	4320.0	4320.0	0.0	16200.0	16200.0	INF
1959	24700.0	24700.0	24700.0	24700.0	16200.0	INF

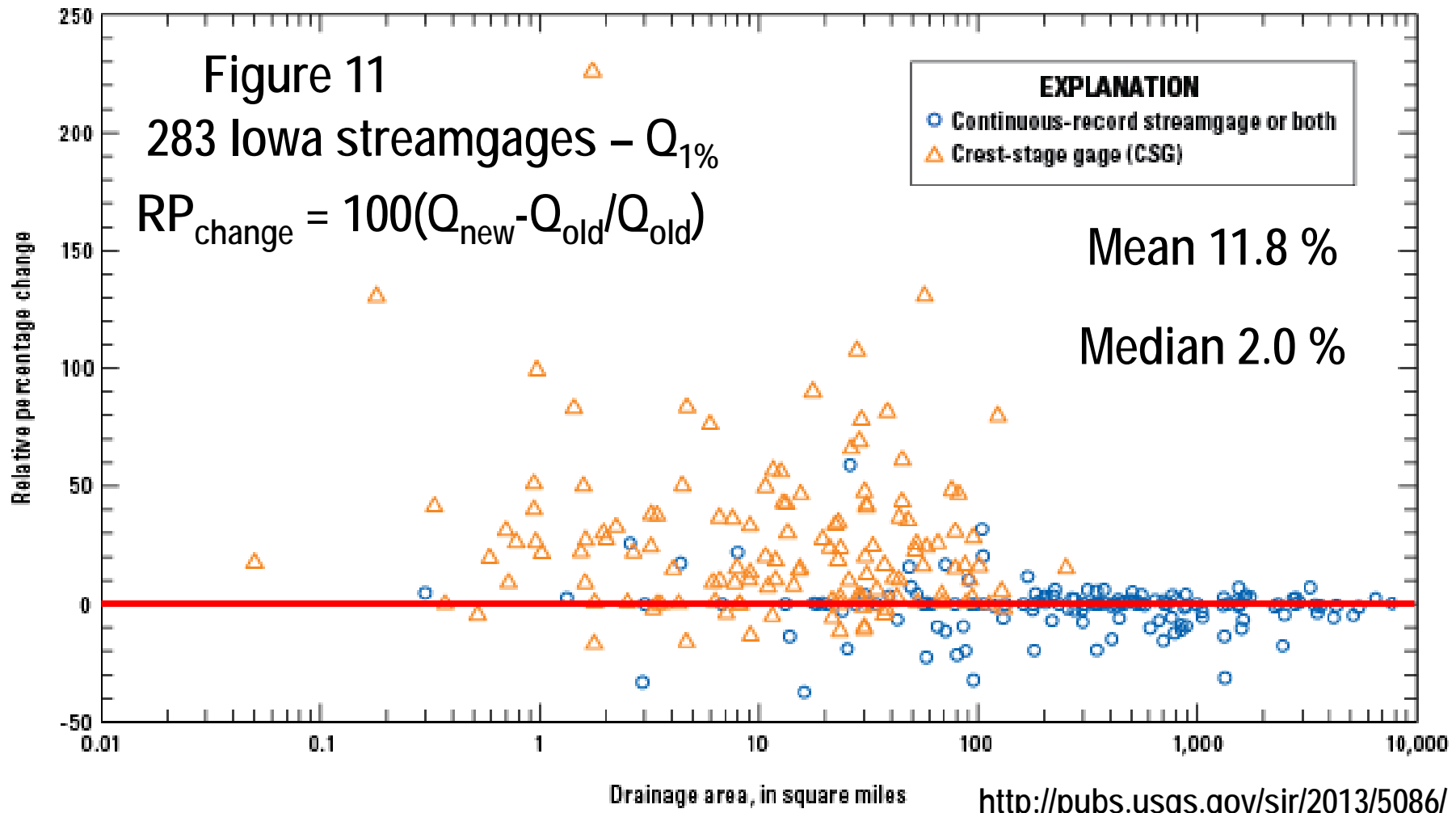
Historic Period

PILF

MGB  
threshold =  
16,200 cfs

PILFs

# Relative % change between EMA/MGB & B17B/GB using new skew for $Q_{1\%}$



# Final flood-frequency estimates for streamgages computed using WIE

$$\log(\hat{Q}) = \frac{\text{MSE}[\log(Q_{re})] \cdot \log(Q_{as}) + \text{MSE}[\log(Q_{as})] \cdot \log(Q_{re})}{\text{MSE}[\log(Q_{as})] + \text{MSE}[\log(Q_{re})]}$$

Weighted Independent Estimates from appendix 8 of Bulletin 17B

B17B/EMA and regional regression equation

estimates are considered independent estimates

WIE program uses the variance and estimate of the B17B/EMA

flood-frequency analysis and the variance and estimate of the

regional regression equation to compute a weighted flood-frequency

estimate and variance at a streamgage

# WIE estimates for Cedar River at Waterloo streamgauge 05464000

05464000					
Annual exceedance probability of instantaneous peak discharges, based on WIE method, EMA/MGB analysis computed using a historical period length of 85 years (1929-2013)					
Annual exceedance probability	Recurrence interval (years)	Discharge (ft <sup>3</sup> /s)	95% lower confidence interval (ft <sup>3</sup> /s)	95% upper confidence interval (ft <sup>3</sup> /s)	
0.500	2	25,800	22,400	29,700	
0.200	5	43,200	37,500	49,900	
0.100	10	55,000	47,100	64,300	
0.040	25	71,100	59,200	85,400	
0.020	50	82,800	67,100	102,000	
0.010	100	94,100	73,900	120,000	
0.005	200	110,000	83,200	145,000	
0.002	500	123,000	88,900	170,000	
KENTAU statistic		0.008			
P-level		0.924			
Begin year		1941			
End year		2013			
Number of peaks		73			



EMA/MGB  $Q_{1\%} = 101,000$  cfs  
variance of estimate = 0.0041

RRE  $Q_{1\%} = 80,400$  cfs  
variance of estimate = 0.0090

WIE  $Q_{1\%} = 94,100$  cfs  
variance of estimate = 0.0029

# QUESTIONS